



Effect of FRP Strips Configuration, Beam Dimensions and Amount of Tensile Rebars on Shear Capacity of Reinforced Concrete Beams

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ABSTRACT: In this paper, the effect of the distance, number of layers and cross-sectional area of FRP strips, the amount of longitudinal rebars, dimensions of beam and compressive strength of concrete on shear capacity of reinforced concrete beams with rectangular cross-section stiffened by FRP strips under symmetrical concentrated loads using the finite element method has been studied. For this reason, the non-linear analysis of 101 reinforced beams has been performed for evaluation of the effect of the parameters on load capacity and mid-span deflection of the beams with and without strengthening. Obtained results indicate that for a constant concrete compressive strength, increasing the width and the number of layers of FRP strips increases the load capacity compared to the control beam. By changing the layout of reinforcing strips with irregular intervals along the beam, the load capacity increase is about 6% to 35%. Also, the increase of the amount of longitudinal rebars from $\Phi 10$ to $\Phi 14$ increasing the compressive strength of the concrete from 30 MPa to 50 MPa, and increasing the cross-sectional area of the beam from 150×300 mm to 150×400 mm in unstiffened beams, increase the load capacity by 31%, 23% and 55%, respectively.

1-Introduction

Reinforced concrete beams are important elements for the performance and safety of the structures. The use of FRP strips is a suitable strengthening method for improving the performance. A number of researchers have studied the effects of FRP strips on structural behaviour of reinforced concrete beams [1-3].

The objective of this research is the study of the effect of the distance, number of layers and cross-sectional area of FRP strips, the amount of longitudinal rebars, dimensions of the beam and compressive strength of the concrete on the shear capacity of reinforced concrete beam with rectangular cross-section under symmetrical concentrated loads.

Analysis has been performed using the finite element code ABAQUS assuming non-linear behaviour for materials.

2-Methodology

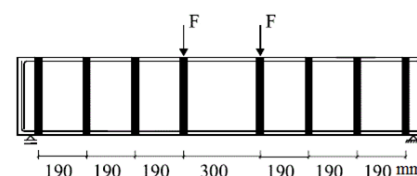
Analysed models are divided into a number of groups. In all three groups D, E and F the amount of compressive steel rebar is $2\Phi 6$. Yield stress and ultimate tensile stress of compressive rebars are $f_y=622$ MPa and $f_u=702$ MPa, respectively. Each FRP strip has a thickness of 0.167 mm, ultimate tensile strength of $f_{fu}=3000$ MPa and modulus of elasticity of $E_f=390000$ MPa. Letters J, K and L relate respectively to the strip widths of 25 mm, 35 mm and 45 mm. M, N and O to strips with 1, 2 and 3 layers, and P, Q and R to 8, 10 and

14U-shaped strips along the beam.

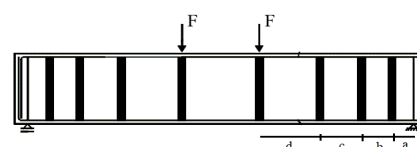
As shown in Figure 1a, the beam is simply supported with two symmetrical concentrated loads, which are increased until the beam fails.

The distance between U-shaped strips is varying and represented by a, b, c and d as presented Figure 1b. For simplicity, models are put into two groups S and T. The amount of steel rebars and concrete compressive strength are represented by D and H.

Figure 1. a) Studied beam, b) distance parameters for U-shaped strips



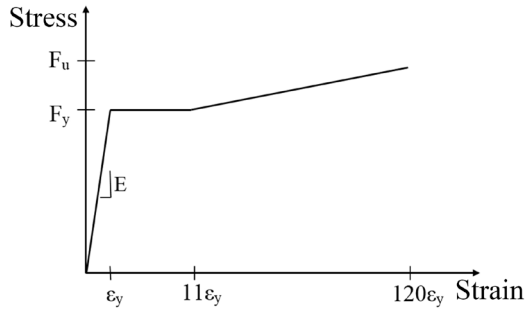
(a)



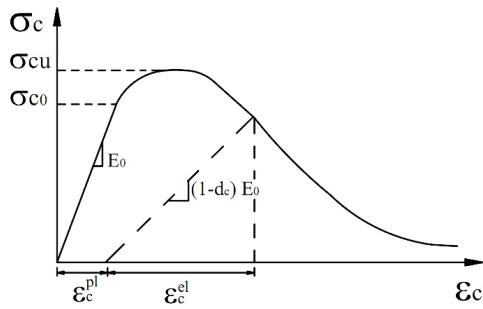
(b)

The von Mises yield criterion [4] and three-line curve stress-strain relationship (Figure 2a) [5] have been used for steel. The damaged plasticity model [6] has been considered for concrete (Figures 2b and c). For FRP strips the Tsai-Wu failure criterion [7] has been used. The effect of parameters has been evaluated on shear capacity and mid span deflection of the beam.

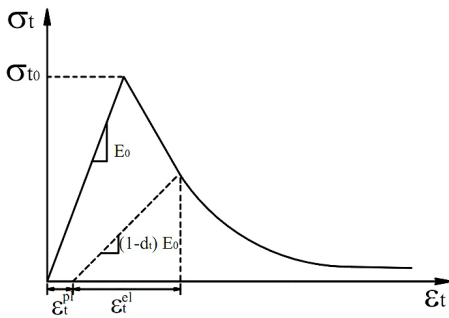
Figure 2. a) Stress-strain relationship: a) steel [4], b) compression for concrete damaged plasticity model [5], c) tension for concrete damaged plasticity model [6]



(a)

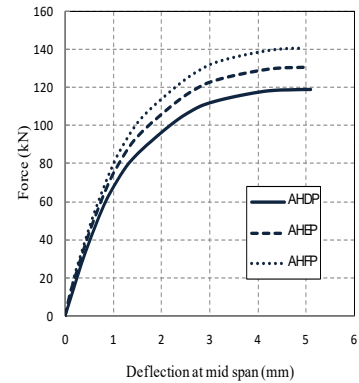


(b)

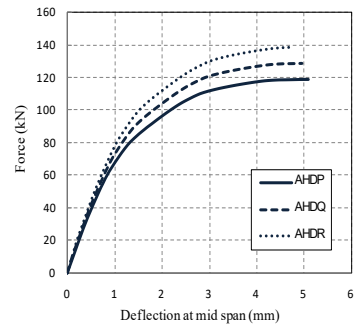


(c)

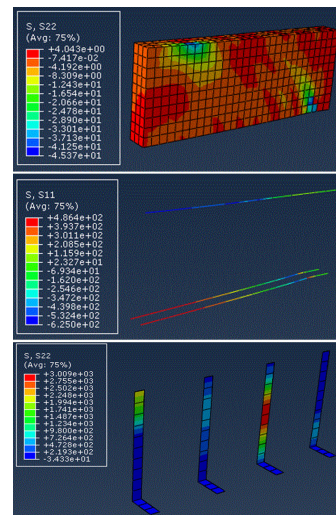
Figure 3. a) Load-deflection diagram: a) with different steel types, b) with different number of FRP strips, c) stress contour in concrete, steel rebars and FRP strips



(a)



(b)



(c)

3-Parametric Study

Figure 3 shows the load-deflection diagrams at the mid span of the beams for group A and stress contour. Figure 3a is for steel types of D, E and F and Figure 3b is for FRP strip types of P, Q and R.

The maximum shear capacity is for beam AHFP with a final load of 140.66 kN and then for beams AHEP and AHDP with final loads of 130.83 kN and 118.83 kN, respectively. Similar results are obtained for beam groups B and C. Therefore, for beams with steel type F and constant concrete com-

pressive strength type H, shear capacity is more than beam types of E and D, and the failure mode changes from shear (Figure 3a) to bending.

4-Conclusions

Main conclusions of the research are as follows.

1. Larger values of concrete compressive strength result in increasing the shear capacity up to 45% and decreasing the mid span deflection.

2. By increasing the width of FRP strips the shear capacity increases by 10%. Increasing the number of FRP strip layers increases the shear capacity by 18% and increases the mid span deflection.

3. It is possible to increase the shear capacity by 35% by changing the distance between the FRP strips.

4. The shear capacity increases by increasing the number of FRP strips.

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